

EWPAA

COMMERCIAL & INDUSTRIAL FLOORING DESIGN

Everything you need to know about installing plywood as a flooring system for commercial and industrial applications

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Introduction

EWPAA structural plywood flooring has proved to be both safe and reliable when used in commercial and industrial buildings, and is easily designed to resist the high loading criteria required by the building codes. Its resilience and strength have been utilised in storage decks, truck floors and flooring in sporting complexes and agricultural buildings. Plywood flooring systems are a cost effective alternative to suspended floors in such applications.

EWPAA/JAS-ANZ branded 'Tested Structural' plywood is a fully engineered structural product manufactured to AS/NZS 2269 "Plywood-structural" and is the only panel included in AS1720.1 "Timber Structures: Design Methods" which is often referred to as "the Timber Design Code".

The key to structural plywood's properties is that its manufacture involves cross-laminating timber veneers together using the permanent Type A phenolic bond.



The cross-lamination utilises timber's superior properties along the grain enabling stresses to be distributed in all directions in the panel. As structural plywood branded with the EWPAA/JAS-ANZ 'Tested Structural' stamp is manufactured under the Engineered Wood Products Association of Australasia's third party audited quality control program, the performance and structural characteristics of the product is assured.

This design manual details the physical, mechanical and structural properties of the standard range of EWPAA/JAS-ANZ branded structural plywood, followed by load/span tables and the installation requirements for plywood flooring.

Benefits of structural plywood flooring

- Safe and Reliable The EWPAA quality control program ensures safe and reliable panels with compliance to relevant Australian Codes.
- Durability the permanent Type A bond has been proven to perform structurally for at least 50 years.
- Assured Load Capacity As structural plywood has standardised and reliable structural properties, it can
 be designed to meet the high concentrated loads specified for commercial and industrial flooring in the
 Timber Design Code. Additionally, the floor can be designed as a structural diaphragm to transfer lateral
 loads within a structure.
- **Dimensional Stability** Plywood's cross- laminated construction provides excellent dimensional stability under changes of moisture content. This is particularly advantageous if the flooring is exposed to the weather during installation.
- **Light Weight** Plywood's superior strength and stiffness for its weight results in light-weight panels that are easy to install.
- **Impact Resistant** Plywood's cross-laminated construction makes panels highly resistant to edge and impact damage. Additionally, fatigue from cyclic loads is not a problem.
- **Environmentally Friendly** Plywood is made from a natural product, wood, which is a renewable resource, and requires limited use of energy in its manufacture.
- Chain of Custody Certification is available –
 Chain of Custody (CoC) basically tracks a wood or forest product from its origin in a certified forest through to its end use as a wood or forest product by the consumer. CoC covers all intermediate steps such as harvesting, transportation, primary and secondary processing, manufacturing, re-manufacturing, distribution and sales.



Product details

To comply with the recommendations of this manual, the structural plywood must be branded with the EWPAA/JAS-ANZ 'Test Structural' stamp. The EWPAA/JAS-ANZ stamp ensures that the plywood has been manufactured under the EWPAA's third party audited, process control based, quality control program and assures compliance with Australian Standards AS/NZS 2269.0-2008 "Plywood-Structural Specifications" and AS1720.1-1997 "Timber Structures: Design Methods".



Tongued and Grooved Edges

Structural plywood is supplied with square edges, however, it may be specified with a tongue and groove along the length of the panel. Based on full scale testing the plastic T&G joint has a design capacity of 7.5kN concentrated imposed (live) load.

In applications where the concentrated load is 7.5kN or less, the T&G joint eliminates the need for nogging. Please note panel end joints must be supported by a sub-floor member. For design concentrated loads exceeding 7.5kN the T and G joint must be supported by a designed nogging.

Dimensions and Tolerances

The most common panel size of structural plywood is 2400 x 1200mm, while 2700 x 1200mm is becoming more widely available.

T&G panels 2250 x 1200mm for 450mm spaced joists are available in thicknesses to suit residential loadings. Other structural plywood panel sizes available on special order are :

- 2400mm x 900mm
- 1800mm x 1200mm
- 1800mm x 900mm
- 3000mm x 1200mm

The first dimension is the length of the panels along the face grain, the second being across the grain. The tolerance on length and width is ± 1.5 mm.

The standard structural plywood thicknesses for flooring are 12, 15, 17, 19, 21, 22, 25. Thicknesses over 25mm are normally against order. It is best to check both the thicknesses and panel sizes available through EWPAA plywood merchants. Thickness tolerances for structural plywood are:

Thickness	Tolerance
Sanded sheets over 7.5 mm thick up to and including 17.5 mm	±4%.
Sanded sheets over 17.5 mm thick up to and including 25 mm	±3%.
Un-sanded sheets	an additional thickness tolerance of +0.3 mm per sheet.

NOTE: For sanded sheets over 25 mm thick, the tolerance on thickness should be agreed between the manufacturer and the purchaser.

Tongued and grooved structural plywood is generally available in a range of panel sizes and thicknesses to suit residential dwelling applications.

Face and Back Grades

The standard face grade for structural plywood for flooring is 'C' which is a non-appearance grade with a solid surface. The standard grade for the back of the panel is 'D'. The D grade surface can contain open defects such as knot holes and splits. The effect of the natural defects has been taken into account in the derivation of characteristic strengths and elastic modulii. The C grade face on structural plywood is suitable for direct covering with floor coverings such as vinyl.

Plywood Characteristics

Durability

All structural plywood manufactured to AS/NZS 2269.0 has the permanent Type A 'marine' bond. This bond is achieved using a phenolic adhesive which is distinctly dark in colour. The standard bond test for Type A gluelines is a 72 hour continuous boil in water, after which the plywood veneers are chiselled apart at the gluelines. In a well-made glueline, separation of the plies occurs through breaking of the wood itself and not by separation of the plies along the gluelines.

The pass requirement is at least 50% of the failure, after chiselling, is in the wood fibre (which indicates that the bond is at least as strong as the wood). This test for phenolic bonds has been proven to simulate over half a century of actual exposure without glueline breakdown



Structural Plywood Flooring, Clear Finished, Shop Floor

Standard structural plywood flooring is not recommended for floors or decks permanently exposed to the weather. If permanent weather exposure is required contact the EWPAA for additional product and installation data

Formaldehyde Emission

Formaldehyde is a colourless strong smelling gas. Formaldehyde occurs naturally in the environment and is emitted by processes such as combustion, decay and is emitted naturally by all timber species. The International Agency for Research on Cancer (IARC) a division of the World Health Organization has recently reclassified formaldehyde from a group 2A suspected carcinogen to a known carcinogen. It must be stressed that the cancer causing properties of formaldehyde are only evident at **very high** concentrations, hundreds of times greater than levels emitted from structural plywood.

The formaldehyde emission classes in the Australian/New Zealand Standards are detailed below. Products labelled with emission classes E_0 and E_1 , have extremely low formaldehyde emissions.

Formaldehyde Emission	Classes from	Australian /	New Zealand Standards
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Emission Class	Formaldehyde Emission Limit (mg/l)	Formaldehyde Emission Limit (ppm)*
E _o	Less than or equal to 0.5	Less than or equal to 0.041
E ₁	Less than or equal to 1.0	Less than or equal to 0.08
E ₂	Less than or equal to 2.0	Less than or equal to 0.16
E ₃	Greater than 2.0	Greater than 0.16

^{*} Based on a test chamber volume of 10 litres, zero airflow during the 24hr test cycle, molecular weight of formaldehyde 30.03 and the number of microlitres of formaldehyde gas in 1 micromole at 101KPa and 298K.

EWPAA certified structural plywood is constructed with an A Bond phenolic glue line which is compliant with the E_0 classification, and well below acceptable exposure limits specified by Workplace Australia and do not constitute a health risk.

Branding

All EWPAA quality controlled structural plywood is branded as follows:

- Manufacturer's Name or Brand
- The word 'Structural'
- Face grade, back grade and bond, e.g. CD-A bond
- Stress grade and Australian Standard AS/NZS 2269.0 e.g. F14 AS/NZS 2269.0
- The panel construction code, e.g. 20-30-7
- The EWPAA/JAS-ANZ 'Tested Structural' stamp
- The Formaldehyde Emission amount, e.g. E₀



Structural Properties

Density

As timber is a natural material and a range of species are used to manufacture plywood the density can vary between panels. As a guide F11 stress grade plywood would average 550kg/m³ while F27 stress grade plywood would average 850kg/m³.

Workability

Structural plywood can be worked with conventional wood working tools. It can be cut, drilled, bored, planed and sanded with ease.

Due to its cross-laminated construction plywood panels can be nailed with hand or power driven nails to within 10mm of the edge without fear of splitting or other edge damage. Other wood based and brittle panel materials do not possess this property. This is a real advantage when fixing the panels to narrow timber sub-floor members as the edge damage factor is negligible.

Additionally, the cross-laminated construction imparts impact resistance to the structural plywood panels. Damage during transport, handling and site installation is therefore minimised. The resistance to handling damage is a real cost benefit.

Moisture Content & Dimensional Stability

Next to the durability of the Type A bond, the panel dimensional stability of structural plywood under moisture content and temperature changes is the most important characteristic. Because of its cross-laminated construction, plywood possesses superior dimensional stability characteristics under changes of moisture content compared to all other timber and wood based panels. **Table 1** details the average hygroscopic movement of structural plywood.

Equally significant is that because the natural wood structure is maintained during plywood manufacture all moisture movements in structural plywood for practical purposes can be considered **reversible**. When large floor

areas are to be laid, the hygroscopic movement of the plywood flooring should be determined over the most likely practical moisture content range that the flooring would normally be subjected to by using the data in **Table 1**. The movement should be allowed for in the flooring design by leaving small movement joints at panel perimeters. For small floor areas, the gaps left between panels when laid by hand should accommodate normal hygroscopic movement.

Plywood							-		Moisture	Content Chang	е
Thickness (mm)	Thickness Plies (mm)	Direction*	5% - 12%	12% - 17%	17% - Saturation	Average, 5% - Saturation					
12	5	 	0.016 0.021	0.009 0.008	0.006 0.005	0.011 0.011					
15	5	 	0.016 0.022	0.008 0.010	0.004 0.009	0.010 0.013					
17	7	 	0.017 0.022	0.009 0.010	0.005 0.010	0.011 0.014					
22	9	 	0.017 0.018	0.012 0.010	0.004 0.008	0.012 0.014					

*Direction $\,^{\parallel}$ is along the face grain, Direction $\,^{\perp}$ is across the face grain

Table 1 – Movement (%) of structural plywood per % change of moisture content

Example

Determine the hygroscopic expansion across the grain of a 1200mm wide, 17mm thick structural plywood panel, when installed at 10% moisture content and used in a fully exposed application in which the plywood could become fully saturated with water. Assume fibre saturation is 28%.

As the range is from 10% - 28% the correct selection from **Table 1** is from the 'Average' column, and is 0.014% per % change of moisture content.

Total change in moisture content

=28% - 10%

=18%

Movement in mm of 1200mm panel width

 $= (0.014 / 100) \times 1200 \times 18$

=3.0mm

Thermal Expansion

The average coefficient of **thermal expansion** for structural plywood is approximately 4.5×10^{-6} mm/mm/°C as compared with 11×10^{-6} mm/mm/°C for steel.

Stress Grades

Structural plywood is available in a range of stress grades depending on the species and quality of timber veneer used in the assembly. AS/NZS 2269.0 "Plywood-Structural: Specifications" provides for both visual and mechanical stress grading which are both equally reliable. The stress grade is stamped on the back of each panel. The most common stress grades available are F8, F11 and F14, however, structural plywood with higher stress grades, F17, F22, F27 and F34 are available. The combination of stress grade and thickness for each application can be found using the span tables in this manual. For example, in a residential application and 15mm thick panel of F8 structural plywood is **structurally equivalent** as flooring to a 14mm thick panel of F11 structural plywood.

Allowable Strength and Elastic Modulii

Design capacities are obtained by modifying the characteristic capacities by factors appropriate to the service conditions and material type and application. The characteristic strengths and modulii given in **Table 2** are applicable to structural plywood manufactured to the requirements of AS/NZS 2269.0 "Plywood-Structural: Specifications".

Table 2 gives the characteristic strengths and stiffness's for the full range of structural plywood stress grades. The Table is a reprint of Table 4.1 from AS/NZS 2269.0.

		Characteristi	Short duration	Short duration			
Stress Grade	Bending	Bending Tension		ending Tension Panel Shear Compression in the plane of the sheet		average modulus of elasticity MPa	average modulus of rigidity MPa
	(f'b)	(f't)	(f's)	(f'c)	(E)	(G)	
F34	100	60	6.8	75	21 500	1 075	
F27	80	50	6.8	60	18 500	925	
F22	65	40	6.8	50	16 000	800	
F17	50	30	6.8	40	14 000	700	
F14	40	25	6.1	30	12 000	625	
F11	35	20	5.3	25	10 500	525	
F8	25	15	4.7	20	9 100	455	
F7	20	12	4.2	15	7 900	345	

Table 2: Characteristic strengths and stiffness for structural plywood (Moisture content 15% or less).

Note: To establish design values the characteristic strengths and modulii must be modified in accordance with the factors in AS1720.1 "Timber Structures: Design Methods" Code.

Section Properties

The method for determining the section properties, second moment or area (I) and section modulus (Z) is defined in AS1720.1. The method is based on **parallel ply theory**, which assumes veneers with face grain direction parallel to the span are the sole contributors to strength and are the major contributors to stiffness. Veneers with grain direction in the cross direction, i.e. at right angles to the span, are assumed to make no contribution to strength and only a 3% contribution to stiffness.

Table 3 gives the section properties for the standard thicknesses and constructions of structural plywood specified in AS/NZS 2269.0. In the thicker plywoods, some manufacturers supply plywoods of slightly different constructions to those in AS/NZS 2269.0, consequently with differing section properties and load capacities.

Identification	Panel Thickness	Along th	ne Grain	Across the Grain		
Code	(mm)	l*	Z*	I	Z	
12-24-5	12	115	19.0	33	8.3	
13-30-5	13	165	24.5	35	8.3	
15-30-5	15	225	29.5	65	13.0	
17-24-7	17	285	33.5	120	19.0	
19-30-7	19	450	46.5	155	21.5	
20-30-7	20	470	47.0	195	26.5	
21-30-7	21	555	52.5	240	29.5	
21-24-9	21	565	51.5	300	33.5	
25-30-9	25	900	70.5	475	46.5	
26-24-11	26	990	74.0	590	51.5	
27-30-9	27	1110	81.0	580	52.5	
31-24-13	31	1590	100.0	1020	74.0	
33-30-11	33	1940	115.0	1150	81.0	
36-24-15	36	2380	130.0	1630	100.0	
39-30-13	39	3100	155.0	1990	115.0	

*I = Second Moment of Area (mm⁴/mm), Z = Section Modulus (mm³/mm)

Table 3 Second moment of area and section modulii for structural plywoods

The identification code shown in **Table 3** uses the three key variables on which the section properties of structural plywood depend to identify and specify a panel.

The three values are the total thickness of plywood (mm), the actual thickness (after drying and pressing) of the face veneer (mm x 10), and the number of plies in the assembly. For Example, The identification code "21-24-9" would be stamped on a plywood panel that has the following characteristics:

- 21mm total thickness
- 2.4mm face veneer
- 9 plies in the panel

Flooring Design

The flooring and its support structure must be designed to meet the load/deflection criteria specified by the designer. This manual does not cover the design of sub-floor members. Using standard 2400mm long structural plywood panels, joists are best spaced at 400mm or 480mm, or perhaps 600mm. It may be possible for designers to optimise sub-floor member sizes by taking account of the composite stressed skin action achieved by connecting the structural plywood flooring to structural timber or steel joists.



Floor Loadings

Floor imposed (live) loads for a wide range of buildings and occupancies are given in Table 3.1 of AS/NZS 1170-2002 "Structural Design Actions – Part 1: Permanent, Imposed and other actions". The loading requirements of the Building Code of Australia are deemed to be satisfied by using this Code. As designers should make allowance for reasonable future changes to the use of buildings or individual rooms, **Table 4** provides a summary of general load requirements for buildings. It would be reasonable to design the flooring in a commercial building to meet imposed (live) loads of 5kPa and 4.5kN.

Flooring application	Uniformly Distributed Load (kPa)	Concentrated Load (KN)
Residential	1.5	1.8
Assembly Areas	3.0 - 5.0*	2.7 - 3.6
Offices	3.0	2.7*
Retail Sales Areas	4.0	3.6*
Office Storage Space, File Rooms	5.0	4.5
Public Corridors & Spaces	4.0 - 5.0	4.5*
Stages	7.5	4.5
General Storage	2.4 per meter of storage height	7.0*
Drill Rooms and Halls	5.0*	9.0*
Light Vehicle Traffic Areas (< 2,500Kg)	2.5	13*
Medium Vehicle Traffic Areas (> 2,500Kg And < 10,000Kg)	5.0	31*

^{*}to be determined but not less than the given value.

Table 4 Summary of AS/NZS 1170.1 Floor imposed (live) Loads

It can be seen from **Table 8** that structural plywood has excellent distributed load capacity, thus it is obvious after reference to **Table 4**, concentrated imposed (live) loads will almost always control the flooring design process.

Occasionally the high localised concentrated imposed loads typical of small diameter hard wheels can cause localised abrasion of flooring surfaces. Thus it is good practice to use larger diameter and/or softer compound wheels to reduce the compressive stresses normal to the flooring surface.



Design Procedure

Once the allowable imposed (live) loads have been established, the plywood flooring designer has two options. Firstly, for residential type loads up to uniformly distributed imposed loads of 3kPa and concentrated imposed These less conservative and less stiff options have been derived from tests, and **Table 5** is included in AS1684 "Residential timer-framed construction" code. However the recommended option for the more highly loaded commercial and industrial floors requires the use of **Tables 6**, **7**, and **8**.

Structural Plywood	Maximum Joist Spacing							
Thickness (mm)	F8	F11	F14					
12	400	420	440					
13	430	450	480					
14	460	480	510					
15	480	520	540					
16	510	540	570					
17	540	560	600					
18	560	590	620					
19	590	620	660					
20	610	650	680					
21	640	670	710					
22	660	700	740					

Table 5 - Maximum Allowable Joist Spacing for T&G Structural Plywood in Residential Building

Notes

- The above Table is based on tests and is included in Table 5.3 of AS 1684.2 2006 "Residential Timber-framed construction Non-Cyclonic Areas".
- Suitable for occupancy requirements of uniformly distributed loads of up to 3kPa and concentrated loads no greater than 2.7kN.
- The structural plywood face grain runs perpendicular to the joists.
- All of the thicknesses shown are not always necessarily readily available.
- The joist spacing is the centre to centre distance between joists.
- The plywood face veneers must not be thinner than any of the inner veneers.



Fixing requirements

Structural plywood flooring may be fixed to the sub-floor with hand or power driven fasteners or a combination of mechanical fasteners and structural elastomeric adhesive for a more rigid squeak free system.

When mechanical fasteners are used without structural elastomeric adhesive the **recommended fastener spacing** is 150mm centres at panel ends and 300mm centres at intermediate joists. When elastomeric adhesive is used in conjunction with the mechanical fasteners the fastener spacing may be increased to 300mm centres at panel ends and 600mm centres at intermediate joists. Table 9 details the recommended fasteners.

Hand driven nails	2.8mm min. dia. flathead or bullet head nails of length at least 2.5 times plywood thickness
Gun driven nails	2.5mm min. dia gun nails of length at least 2.5 times the plywood thickness
Screws to timber joists	No. 8 x 32mm self drilling countersunk wood screws -up to 20mm plywood No. 10 x 50mm self drilling countersunk wood screws - 21 to 40mm plywood
Screws to steel joists	No. 10 x 50mm countersunk self drilling metal screws - up to 30mm plywood No. 10 x 75mm countersunk self drilling metal screws - 31 to 40mm plywood Note that screws with a "wing tip" may be suitable for screwing to steel joists provided the screws offer the same withdrawal resistance as the equivalent self drilling metal screw.
Adhesives	Structural elastomeric that meets the American Plywood Association Standard AFG- 01 eg H.B. Fuller's 'Max-Bond', or 'Sturdi Bond', or Norton's 'Floormate'

Table 9 – Minimum Fastener Specifications

Notes

- 1. Fastener coatings should be selected to suit for application e.g. hot dip galvanised for chemical storage areas.
- 2. Plywood can be fixed within 10mm of its edges.
- 3. Structural elastomeric adhesive should be used where plywood is fixed to unseasoned timber joists.
- 4. When nailing to seasoned softwood sub-floor members deformed shank nails are recommended.
- 5. To minimise squeaks when fixing plywood to steel joists, the use of structural elastomeric adhesive is strongly recommended.

Load Span Tables

Table 6 has been calculated for concentrated imposed (live) loads in accordance with the Timber Structures Code. The Table has been developed to satisfy the strength requirements of AS/NZS 1170.1 with an overriding serviceability limit of span/200 deflection at the required strength.

Table 7, similarly to **Table 6**, meets the strength requirements of AS/NZS 1170.1 for concentrated imposed loads. However, the span/300 deflection criteria used in **Table 7** will result in a stiffer plywood floor. **Table 7** should be used in preference to **Table 6** where a more stringent stiffness criteria is required. For example, where the flooring is to be subjected to both human and vehicular traffic, or where flooring aesthetics are important.

Finally, **Table 8** provides the uniformly distributed imposed load capacity for structural plywood. The Table is again developed to satisfy the strength criteria of AS/NZS 1170.1 with an overriding serviceability limit of span/200 deflection. This need only be checked for flooring carrying exceptionally high UDL's. In cases where the loading is long term, i.e. permanent load with j_2 =2, then the recommended approach is to double the design load before making the selection from the tabulations.

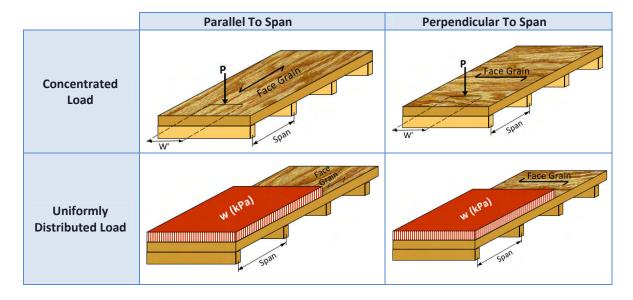
Deflection / Strength Limits

The tables are developed with an overriding strength limit. That is, the tables are developed with the following methodology:

- 1. Determine how much load needs to be applied to the plywood, to cause a deflection of the deflection limit (eg. Span / 300).
- 2. Determine the lower value of the bending moment and shear strength limits.
- 3. If the load from step 1 is less than the load from step 2, display the load from step 1.
- 4. If the load from step 1 exceeds the load from step 2, then display the load from step 2, and colour the cells orange to indicate that a strength limit state was reached before the deflection limit.

Face Grain Direction

The load span tables are divided into "Parallel To Span" and "Perpendicular To Span". This refers to the face grain direction, in relation to the span of the panel:



Design Criteria

- Tables 6, 7 and 8 were calculated using limit state design in accordance with AS1720.1 and using the following assumptions:
 - $k_1 = 0.80$ for uniformly distributed imposed load = 0.94 for concentrated imposed loads, and
 - $J_2 = 1.0$
- The flooring is in a dry interior environment (Moisture Content is less than or equal to 15%).
- The concentration (live) loads are treated as a line load with a distribution width (w') of:
 - o 400mm for 12mm plywood
 - o 450mm for 15, 17 and 19mm plywood
 - o 520mm for 21 and 24mm plywood
 - o 600mm for 26mm and thicker plywood.
- The face grain of the structural plywood runs perpendicular to the joists.
- The structural plywood is continuous over at least two spans.
- All plywood end joints must be supported by a structural member, however nogging is not required to support plastic T&G edge joints for imposed (live) loads up to 7.5kN.
- Concentrated imposed (live) loads, being transient, have a shorter accumulated duration than distributed loads, justifying the differing values for k₁.
- With concentrated loads and UDL's specified in the Structural Design Actions Code, the important criteria is strength, thus a deflection limit of span/200 is reasonable as a minimum acceptable deflection criteria.
- The span/300 criteria for concentrated loads meet the suggested serviceability limit state criteria of AS/NZS1170.0: 2002 "Structural Design Actions", and should be used where a more stringent stiffness criteria is required.

If your design has any of the following:

- A floor that is a primary structural element in a structure intended to fulfil an essential service or post disaster function;
- Imposed loads lasting longer than 5 days (which affects creep and strength); or
- A high moisture content environment, such as an exposed wet area or tropical climate.

You cannot use the following tables!

The "Load Span Table" software can calculate tables for any design requirements, and is available for free download from the <u>EWPAA Web Site</u>. This software is meant for use by experienced professional designers only, and can only be used if this manual has been fully read and understood.

Example Using the Tables

Note, if you need assistance with the basic formulae used in this example, please see the "Basic Formulae" section at the end of this manual.

Design:

Say we are designing a floor, and we know that our worst case loading requirement will be a 4 wheeled trolley that when fully loaded will carry 650kg. Joists are at 450mm centres, and we want to minimise the deflection, so that the trolley moves smoothly over the floor. The plywood panels will be laid across the joists, thus the face grain will be parallel to span.

Solution:

Because we want to minimise the deflection, we will choose a maxim mum deflection of Span / 300, which in this case would yield a 450 / 300 = 1.5mm deflection maximum. As this is our limit, we must use Table 7.

Our Trolley weighs 650kg. Assuming we have relatively small diameter wheels, this can be converted to a maximum point load as follows :

```
Weight / wheel = 650 / 4 = 163 \text{Kg}.
Force / wheel = 163 \times 9.8 = 1.597 \text{ kN, say } 1.6 \text{kN}.
```

Thus, we are designing for a point load of 1.6kN.

We now need to find the plywood panel that will support this weight. This is easily done by looking down the "450" column, until we find a value of 1.6kN. Reading across will give us the correct plywood panel and stress grade.

As an example, the following is an excerpt from table 6, containing the information we want:

			Parallel To Span									
Panel Code	Stress Grade	300	400	450	480	600	800	900				
	F8	2.3	1.3	1.0	0.9	0.6	0.3	0.3				
	F11	2.6	1.5	1.2	1.0	0.7	0.4	0.3				
	F14	3.0	1.7	1.3	1.2	0.8	0.4	0.3				
15-30-5	F17	3.5	2.0	1.6	1.4	0.9	0.5	0.4				
	F22	4.0	2.3	1.8	1.6	1.0	0.6	0.4				
	F27	4.6	2.6	2.1	1.8	1.2	0.7	0.5				
	F34	5.4	3.0	2.4	2.1	1.3	0.8	0.6				

Thus, an F17, 15-30-5 plywood panel will be applicable for our design. Note that there are many other solutions to this example. The solution you choose will depend on plywood availability, and other factors important to you.

Table 6: Allowable Concentrated Imposed (Live) Loads (kN), Deflection Limit of Span/200

				Para	llel To	Span			Perpendicular To Span						
Panel Code	Stress Grade	300	400	450	480	600	800	900	300	400	450	480	600	800	900
	F8	1.6	0.9	0.7	0.6	0.4	0.2		0.5	0.3	0.2				
	F11	1.8	1.0	8.0	0.7	0.4	0.3		0.5	0.3	0.2	0.2			
	F14	2.1	1.2	0.9	0.8	0.5	0.3	0.2	0.6	0.3	0.3	0.2			
12-24-5	F17	2.4	1.3	1.1	0.9	0.6	0.3	0.3	0.7	0.4	0.3	0.3			
	F22	2.7	1.5	1.2	1.1	0.7	0.4	0.3	0.8	0.4	0.4	0.3			
	F27	3.2	1.8	1.4	1.2	0.8	0.4	0.4	0.9	0.5	0.4	0.4	0.2		
	F34 F8	3.7 2.7	2.1 1.9	1.6 1.5	1.4	0.9	0.5	0.4	1.1	0.6	0.5	0.4	0.3		
	F11	3.8	2.2	1.8	1.5	1.0	0.6	0.4	1.1	0.6	0.4	0.4	0.2		
	F14	4.4	2.5	2.0	1.8	1.1	0.6	0.4	1.3	0.0	0.6	0.4	0.3		
15-30-5	F17	5.3	3.0	2.3	2.1	1.3	0.7	0.6	1.5	0.9	0.7	0.6	0.4	0.2	
	F22	6.0	3.4	2.7	2.4	1.5	0.8	0.7	1.7	1.0	0.8	0.7	0.4	0.2	
	F27	7.0	3.9	3.1	2.7	1.7	1.0	0.8	2.0	1.1	0.9	0.8	0.5	0.3	0.2
	F34	8.1	4.5	3.6	3.2	2.0	1.1	0.9	2.3	1.3	1.0	0.9	0.6	0.3	0.3
	F8	3.1	2.3	1.9	1.7	1.1	0.6	0.5	1.7	1.1	0.8	0.7	0.5	0.3	0.2
	F11	4.3	2.8	2.2	2.0	1.3	0.7	0.6	2.2	1.2	1.0	0.8	0.5	0.3	0.2
	F14	4.9	3.2	2.5	2.2	1.4	0.8	0.6	2.5	1.4	1.1	1.0	0.6	0.3	0.3
17-24-7	F17	6.2	3.8	3.0	2.6	1.7	0.9	0.7	2.9	1.6	1.3	1.1	0.7	0.4	0.3
	F22	7.6	4.3	3.4	3.0	1.9	1.1	0.8	3.3	1.8	1.5	1.3	0.8	0.5	0.4
	F27	8.8	5.0	3.9	3.4	2.2	1.2	1.0	3.8	2.1	1.7	1.5	0.9	0.5	0.4
	F34	10.2	5.8	4.6	4.0	2.6	1.4	1.1	4.4	2.5	2.0	1.7	1.1	0.6	0.5
	F8	3.5	2.6	2.3	2.1	1.4	0.8	0.6	2.4	1.6	1.3	1.1	0.7	0.4	0.3
	F11	4.9	3.6	2.8	2.5	1.6	0.9	0.7	3.4	1.9	1.5	1.3	0.8	0.5	0.4
	F14	5.6	4.1	3.2	2.8	1.8	1.0	0.8	3.9	2.2	1.7	1.5	1.0	0.5	0.4
19-24-7	F17	7.0	4.7	3.8	3.3	2.1	1.2	0.9	4.5	2.5	2.0	1.8	1.1	0.6	0.5
	F22	9.2	5.4	4.3	3.8	2.4	1.4	1.1	5.2	2.9	2.3	2.0	1.3	0.7	0.6
	F27	11.2	6.3	5.0	4.4	2.8	1.6	1.2	6.0	3.4	2.6	2.3	1.5	0.8	0.7
	F34	13.0	7.3	5.8	5.1	3.2	1.8	1.4	6.9	3.9	3.1	2.7	1.7	1.0	0.8
	F8	4.3	3.2	2.9	2.7	1.7	1.0	0.8	2.0	1.3	1.1	0.9	0.6	0.3	0.3
	F11	6.0	4.5	3.5	3.1	2.0	1.1	0.9	2.8	1.6	1.2	1.1	0.7	0.4	0.3
9-30-7	F14 F17	6.9 8.6	5.1 5.9	4.0 4.7	3.5 4.1	2.3	1.3 1.5	1.0	3.2	1.8 2.1	1.4	1.2 1.4	0.8	0.4	0.4
3-00-7	F22	11.2	6.8	5.4	4.7	3.0	1.7	1.3	4.2	2.4	1.9	1.6	1.1	0.6	0.5
	F27	13.0	7.9	6.2	5.5	3.5	2.0	1.6	4.9	2.7	2.2	1.9	1.2	0.7	0.5
	F34	13.0	9.1	7.2	6.3	4.1	2.3	1.8	5.6	3.2	2.5	2.2	1.4	0.8	0.6
	F8	5.6	4.2	3.7	3.5	2.4	1.4	1.1	3.2	2.4	1.9	1.6	1.1	0.6	0.5
	F11	7.8	5.9	5.0	4.4	2.8	1.6	1.3	4.4	2.7	2.2	1.9	1.2	0.7	0.5
	F14	8.9	6.7	5.7	5.0	3.2	1.8	1.4	5.1	3.1	2.5	2.2	1.4	0.8	0.6
21-30-7	F17	11.2	8.4	6.7	5.9	3.8	2.1	1.7	6.3	3.6	2.9	2.5	1.6	0.9	0.7
	F22	14.5	9.7	7.6	6.7	4.3	2.4	1.9	7.4	4.2	3.3	2.9	1.9	1.0	0.8
	F27	16.7	11.2	8.8	7.8	5.0	2.8	2.2	8.6	4.8	3.8	3.3	2.1	1.2	1.0
	F34	16.7	13.0	10.3	9.0	5.8	3.3	2.6	10.0	5.6	4.4	3.9	2.5	1.4	1.1
	F8	8.9	6.7	5.9	5.5	4.4	2.6	2.0	4.7	3.5	3.1	2.9	1.9	1.1	0.9
	F11	12.4	9.3	8.3	7.8	5.3	3.0	2.3	6.6	4.9	4.0	3.5	2.2	1.3	1.0
	F14	14.2	10.6	9.5	8.9	6.0	3.4	2.7	7.5	5.6	4.5	4.0	2.6	1.4	1.1
25-30-9	F17	17.8	13.3	11.8	11.0	7.0	3.9	3.1	9.4	6.7	5.3	4.7	3.0	1.7	1.3
	F22	22.9	17.3	14.2	12.5	8.0	4.5	3.6	12.2	7.7	6.1	5.3	3.4	1.9	1.5
	F27	22.9	20.8	16.5	14.5	9.3	5.2	4.1	15.0	8.9	7.0	6.2	3.9	2.2	1.7
	F34	22.9	22.9	19.1	16.8	10.8	6.1	4.8	18.3	10.3	8.1	7.1	4.6	2.6	2.0
	F8	10.0	7.5	6.6	6.2	5.0	3.2	2.5	6.4	4.8	4.3	4.0	3.0	1.7	1.3
	F11	14.0	10.5	9.3	8.7	6.5	3.7	2.9	9.0	6.8	6.0	5.3	3.4	1.9	1.5
27 20 0	F14	15.9	11.9	10.6	9.9	7.4	4.2	3.3	10.3	7.7	6.8	6.1	3.9	2.2	1.7
27-30-9	F17	19.9	14.9	13.3	12.4	8.7	4.9	3.8	12.9	9.6	8.1	7.1	4.5	2.6	2.0
	F22	24.7	19.4	17.3	15.5	9.9	5.6	4.4	16.8	11.7	9.2	8.1	5.2	2.9	2.3
	F27	24.7	23.9	20.3	17.9	11.4	6.4	5.1	20.6	13.5	10.7	9.4	6.0	3.4	2.7
	F34 F8	24.8 14.2	24.7 10.7	23.6 9.5	20.8 8.9	13.3 7.1	7.5 5.3	5.9 4.4	24.7	15.7 7.5	12.4 6.6	10.9	7.0 5.0	3.9	3.1 2.6
	F8 F11	19.9	14.9	13.3	12.4	9.9	6.4	5.0	10.0	10.5	9.3	8.7	6.7	3.8	3.0
	F11	22.8	17.1	15.2	14.2	11.3	7.3	5.8	15.9	11.9	10.6	9.9	7.7	4.3	3.4
33-30-11	F14	28.5	21.3	18.9	17.8	14.2	8.5	6.7	19.9	14.9	13.3	12.4	9.0	5.0	4.0
-0-00-11	F17	30.3	27.7	24.6	23.1	17.3	9.7	7.7	25.9	19.4	17.2	16.0	10.3	5.8	4.6
	1 44	50.5	21.1			17.3	5.1	1.1							
	F27	30.3	30.2	30.2	28.4	20.0	11.2	8.9	30.2	23.9	21.1	18.5	11.9	6.7	5.3

For Single Spans, reduce the allowable concentrated loads by 40%

This table is recommended for use in those applications where floor stiffness is not critical for flooring aesthetics and/or general usage. Table 7 should be used where a more stringent stiffness criteria is required.

Table 7 : Allowable Concentrated Imposed (Live) Loads (kN), Deflection Limit of Span/300

				Para	llel To	Span					Р	erpend	dicular [*]	To Spa	n	
Panel Code	Stress Grade	300	400	450	480	600	800	900	30	00	400	450	480	600	800	900
12-24-5	F8	1.0	0.6	0.5	0.4	0.3			0.							
	F11	1.2	0.7	0.5	0.5	0.3			0.:							
	F14	1.4	0.8	0.6	0.5	0.3			0.4		0.2					
	F17	1.6	0.9	0.7	0.6	0.4	0.2	0.0	0.		0.3	0.2	0.0			
	F22 F27	1.8 2.1	1.0 1.2	0.8	0.7	0.5 0.5	0.3	0.2	0.		0.3	0.2	0.2			
	F34	2.5	1.4	1.1	1.0	0.6	0.3	0.2	0.		0.4	0.3	0.2			
	F8	2.3	1.3	1.0	0.9	0.6	0.3	0.3	0.	_	0.4	0.3	0.3			
15-30-5	F11	2.6	1.5	1.2	1.0	0.7	0.4	0.3	0.		0.4	0.3	0.3			
	F14	3.0	1.7	1.3	1.2	0.8	0.4	0.3	0.9		0.5	0.4	0.3	0.2		
	F17	3.5	2.0	1.6	1.4	0.9	0.5	0.4	1.0	0	0.6	0.5	0.4	0.3		
	F22	4.0	2.3	1.8	1.6	1.0	0.6	0.4	1.3	2	0.7	0.5	0.5	0.3		
	F27	4.6	2.6	2.1	1.8	1.2	0.7	0.5	1.3	3	8.0	0.6	0.5	0.3		
	F34	5.4	3.0	2.4	2.1	1.3	0.8	0.6	1.0		0.9	0.7	0.6	0.4	0.2	
	F8	2.9	1.6	1.3	1.1	0.7	0.4	0.3	1.3		0.7	0.6	0.5	0.3		
	F11	3.3	1.9	1.5	1.3	8.0	0.5	0.4	1.4		8.0	0.6	0.6	0.4	0.2	
4= 0	F14	3.8	2.1	1.7	1.5	0.8	0.5	0.4	1.0		0.9	0.7	0.6	0.4	0.2	
17-24-7	F17	4.4	2.5	2.0	1.7	1.1	0.6	0.5	1.9		1.1	0.9	0.7	0.5	0.3	0.2
	F22	5.1	2.9	2.3	2.0	1.3	0.7	0.6	2.:		1.2	1.0	0.9	0.5	0.3	0.2
	F27	5.9	3.3	2.6	2.3	1.5	0.8	0.7	2.		1.4	1.1	1.0	0.6	0.4	0.3
	F34	6.8	3.8 2.1	3.0 1.6	2.7 1.4	1.7 0.9	1.0	0.8	2.9		1.7	1.3	1.1	0.7	0.4	0.3
19-24-7	F8 F11	3.5 4.2	2.1	1.6	1.4	1.1	0.6	0.4	2.		1.1	0.9 1.0	0.8	0.6	0.3	
	F11	4.2	2.4	2.1	1.9	1.1	0.6	0.5	2.		1.4	1.1	1.0	0.6	0.3	0.3
	F17	5.6	3.2	2.5	2.2	1.4	0.7	0.6	3.		1.7	1.3	1.2	0.8	0.4	0.3
	F22	6.4	3.6	2.9	2.5	1.6	0.9	0.7	3.4		1.9	1.5	1.3	0.9	0.5	0.4
	F27	7.4	4.2	3.3	2.9	1.9	1.0	0.8	4.		2.2	1.8	1.6	1.0	0.6	0.4
	F34	8.6	4.9	3.8	3.4	2.2	1.2	1.0	4.		2.6	2.1	1.8	1.2	0.6	0.5
	F8	4.3	2.6	2.0	1.8	1.1	0.6	0.5	1.0	_	0.9	0.7	0.6	0.4	0.2	0.0
19-30-7	F11	5.3	3.0	2.3	2.1	1.3	0.7	0.6	1.8		1.0	0.8	0.7	0.5	0.3	0.2
	F14	6.0	3.4	2.7	2.4	1.5	0.8	0.7	2.	1	1.2	0.9	0.8	0.5	0.3	0.2
	F17	7.0	4.0	3.1	2.8	1.8	1.0	0.8	2.	5	1.4	1.1	1.0	0.6	0.3	0.3
	F22	8.1	4.5	3.6	3.1	2.0	1.1	0.9	2.	8	1.6	1.2	1.1	0.7	0.4	0.3
	F27	9.3	5.2	4.1	3.6	2.3	1.3	1.0	3.:	2	1.8	1.4	1.3	0.8	0.5	0.4
	F34	10.8	6.1	4.8	4.2	2.7	1.5	1.2	3.	_	2.1	1.7	1.5	0.9	0.5	0.4
	F8	5.6	3.7	2.9	2.5	1.6	0.9	0.7	2.		1.6	1.2	1.1	0.7	0.4	0.3
	F11	7.5	4.2	3.3	2.9	1.9	1.1	0.8	3.		1.8	1.4	1.3	0.8	0.5	0.4
21-30-7	F14	8.6	4.8	3.8	3.4	2.2	1.2	1.0	3.		2.1	1.6	1.4	0.9	0.5	0.4
	F17	10.0	5.6	4.5	3.9	2.5	1.4	1.1	4.		2.4	1.9	1.7	1.1	0.6	0.5
	F22	11.5	6.5	5.1	4.5	2.9	1.6	1.3	4.9		2.8	2.2	1.9	1.2	0.7	0.5
	F27 F34	13.3 15.4	7.5 8.7	5.9 6.9	5.2 6.0	3.3	1.9	1.5 1.7	5.		3.2 3.7	2.5 2.9	2.2	1.4 1.7	0.8	0.6
	F34 F8	8.9	6.6	5.4	4.7	3.9	1.7	1.7	4.	_	2.9	2.9	2.0	1.7	0.9	0.7
	F11	12.4	7.9	6.2	5.5	3.5	2.0	1.6	6.0		3.4	2.6	2.3	1.5	0.7	0.6
	F14	14.2	9.0	7.1	6.3	4.0	2.3	1.8	6.		3.8	3.0	2.7	1.7	1.0	0.7
25-30-9	F17	17.8	10.5	8.3	7.3	4.7	2.6	2.1	7.9		4.5	3.5	3.1	2.0	1.1	0.9
	F22	21.4	12.0	9.5	8.3	5.3	3.0	2.4	9.		5.1	4.0	3.5	2.3	1.3	1.0
	F27	22.9	13.9	11.0	9.6	6.2	3.5	2.7	10		5.9	4.7	4.1	2.6	1.5	1.2
	F34	22.9	16.1	12.8	11.2	7.2	4.0	3.2		2.2	6.9	5.4	4.8	3.0	1.7	1.4
27-30-9	F8	10.0	7.5	6.6	5.9	3.8	2.1	1.7	6.4	4	4.4	3.5	3.1	2.0	1.1	0.9
	F11	13.9	9.7	7.7	6.8	4.3	2.4	1.9	9.	0	5.1	4.0	3.6	2.3	1.3	1.0
	F14	15.9	11.1	8.8	7.7	4.9	2.8	2.2	10	0.3	5.8	4.6	4.1	2.6	1.5	1.2
	F17	19.9	13.0	10.3	9.0	5.8	3.2	2.6	12		6.8	5.4	4.7	3.0	1.7	1.3
	F22	24.7	14.8	11.7	10.3	6.6	3.7	2.9		3.9	7.8	6.2	5.4	3.5	1.9	1.5
	F27	24.7	17.2	13.6	11.9	7.6	4.3	3.4		6.0	9.0	7.1	6.3	4.0	2.3	1.8
	F34	24.7	19.9	15.8	13.9	8.9	5.0	3.9		3.6	10.5	8.3	7.3	4.7	2.6	2.1
	F8	14.2	10.7	9.5	8.9	6.6	3.7	2.9		0.0	7.5	6.6	6.1	3.9	2.2	1.7
	F11	19.9	14.9	13.3	11.8	7.6	4.3	3.4		3.9	10.1	8.0	7.0	4.5	2.5	2.0
00 00 44	F14	22.8	17.1	15.1	13.5	8.6	4.9	3.8	15		11.5	9.1	8.0	5.1	2.9	2.3
33-30-11	F17	28.5	21.3	17.9	15.8	10.1	5.7	4.5		9.9	13.5	10.6	9.4	6.0	3.4	2.7
	F22	30.2	25.9	20.5	18.0	11.5	6.5	5.1	25		15.4	12.2	10.7	6.8	3.8	3.0
	F27	30.2	30.0	23.7	20.8	13.3	7.5	5.9).2	17.8	14.1	12.4	7.9	4.4	3.5
	F34	30.3	30.2	27.5	24.2	15.5	8.7	6.9	30).2	20.7	16.3	14.4	9.2	5.2	4.1

For Single Spans, reduce the allowable concentrated loads by 40%

Table 7 is recommended for calculated vehicular loads where human reactions to vehicular traffic are considered significant.

Table 8 :
Allowable Uniformly Distributed Imposed (Live) Loads (kPa),
Deflection Limit of Span/200

		Parallel To Span				Perpendicular To Span										
Panel Code	Stress Grade	300	400	450	480	600	800	900		300	400	450	480	600	800	900
12-24-5	F8	18.0	10.1	8.0	7.0	4.5	1.9	1.3		7.9	4.4	3.1	2.5	1.3	0.5	0.4
	F11	25.2	14.2	11.2	9.8	5.2	2.2	1.5		11.0	5.1	3.6	2.9	1.5	0.6	0.4
	F14 F17	28.8 36.0	16.2 20.2	12.8 16.0	11.2 13.5	5.9 6.9	2.5 2.9	1.8 2.0		12.6 15.7	5.8 6.8	4.1 4.7	3.3	1.7 2.0	0.7	0.5
	F17	46.8	26.3	18.7	15.4	7.9	3.3	2.0		18.3	7.7	5.4	4.5	2.3	1.0	0.6 0.7
	F27	49.5	30.7	21.6	17.8	9.1	3.8	2.7		21.1	8.9	6.3	5.2	2.6	1.1	0.8
	F34	49.5	35.7	25.1	20.7	10.6	4.5	3.1		24.6	10.4	7.3	6.0	3.1	1.3	0.9
	F8	28.1	15.8	12.5	11.0	7.0	3.7	2.6		12.3	6.9	5.4	4.8	2.5	1.1	0.8
	F11	39.4	22.1	17.5	15.4	9.8	4.3	3.0		17.2	9.6	6.9	5.7	2.9	1.2	0.9
	F14	45.0	25.3	20.0	17.5	11.2	4.9	3.4		19.7	11.0	7.9	6.5	3.3	1.4	1.0
15-30-5	F17	56.3	31.6	25.0	21.9	13.5	5.7	4.0		24.6	13.2	9.3	7.6	3.9	1.6	1.2
	F22 F27	61.8 61.8	41.1 46.3	32.5 40.0	28.5 34.7	15.4 17.8	6.5 7.5	4.6 5.3		32.0 39.4	15.1 17.4	10.6 12.2	8.7 10.1	4.5 5.2	1.9 2.2	1.3 1.5
	F34	61.8	46.4	41.2	38.6	20.7	8.7	6.1		48.0	20.3	14.2	11.7	6.0	2.5	1.8
	F8	31.7	17.8	14.1	12.4	7.9	4.4	3.3		18.0	10.1	8.0	7.0	4.5	2.0	1.4
	F11	44.4	24.9	19.7	17.3	11.0	5.4	3.8		25.2	14.1	11.2	9.8	5.5	2.3	1.6
	F14	50.7	28.5	22.5	19.8	12.6	6.2	4.3		28.8	16.2	12.8	11.2	6.3	2.7	1.9
17-24-7	F17	63.4	35.6	28.1	24.7	15.8	7.2	5.1		36.0	20.2	15.9	14.0	7.3	3.1	2.2
	F22	70.1	46.3	36.6	32.1	19.5	8.2	5.8		46.8	26.3	19.9	16.4	8.4	3.5	2.5
	F27	70.1	52.5	45.0	39.6	22.5	9.5	6.7		57.6	32.3	23.0	19.0	9.7	4.1	2.9
	F34 F8	70.1 36.1	52.5	46.7 16.0	43.8 14.1	26.2 9.0	11.1 5.0	7.8 4.0		70.0 24.9	38.1 14.0	26.7	22.0 9.7	11.3 6.2	4.8 3.2	3.3 2.2
	F11	50.6	20.3 28.4	22.4	19.7	12.6	6.8	4.0		34.9	19.6	11.1 15.5	13.6	8.6	3.6	2.6
19-24-7	F14	57.8	32.5	25.7	22.5	14.4	7.8	5.5		39.9	22.4	17.7	15.5	9.9	4.2	2.9
	F17	72.2	40.6	32.1	28.2	18.0	9.1	6.4		49.9	28.0	22.1	19.4	11.5	4.9	3.4
	F22	78.3	52.8	41.7	36.6	23.4	10.4	7.3		64.8	36.4	28.8	25.3	13.2	5.6	3.9
	F27	78.3	58.7	51.3	45.1	28.5	12.0	8.5		78.3	44.8	35.4	29.8	15.2	6.4	4.5
	F34	78.3	58.7	52.2	48.9	33.1	14.0	9.8		78.3	56.1	42.0	34.6	17.7	7.5	5.2
19-30-7	F8	44.1	24.8	19.6	17.2	11.0	6.1	4.8		20.6	11.6	9.1	8.0	5.1	2.6	1.8
	F11 F14	61.1 70.3	34.7 39.6	27.4 31.3	24.1 27.5	15.4 17.6	8.5 9.8	6.0 6.9		28.8 32.9	16.2 18.5	12.8 14.6	11.2 12.8	7.1 8.1	3.0	2.1 2.4
	F17	78.3	49.5	39.1	34.4	22.0	11.4	8.0		41.2	23.1	18.2	16.0	9.4	4.0	2.8
	F22	78.4	58.7	50.9	44.7	28.6	13.0	9.2		53.5	30.0	23.7	20.8	10.7	4.5	3.2
	F27	78.4	58.7	52.2	48.9	35.2	15.1	10.6		65.9	37.0	29.2	24.3	12.4	5.2	3.7
	F34	78.4	58.7	52.2	48.9	39.1	17.5	12.3		78.3	46.2	34.2	28.2	14.4	6.1	4.3
	F8	49.6	27.9	22.0	19.3	12.4	6.9	5.4		28.1	15.8	12.5	11.0	7.0	3.9	2.8
21-30-7	F11 F14	67.5 77.7	39.0 44.6	30.8 35.2	27.1 30.9	17.3 19.8	9.7 11.1	7.4 8.5		39.4 45.0	22.1 25.3	17.5 20.0	15.3 17.5	9.8 11.2	4.5 5.2	3.2
	F17	86.6	55.7	44.0	38.7	24.7	13.8	9.9		56.3	31.6	25.0	21.9	14.0	6.1	4.2
2.007	F22	86.6	64.9	57.2	50.3	32.1	16.1	11.3		73.2	41.1	32.4	28.5	16.4	6.9	4.9
	F27	86.6	64.9	57.7	54.1	39.6	18.6	13.0		86.5	50.6	39.9	35.1	19.0	8.0	5.6
	F34	86.6	64.9	57.7	54.1	43.2	21.6	15.2		86.5	63.2	49.9	43.0	22.0	9.3	6.5
	F8	68.3	38.4	30.3	26.6	17.0	9.5	7.5		36.1	20.3	16.0	14.1	9.0	5.0	3.9
	F11	80.4	53.7	42.5	37.3	23.8	13.4	10.5		50.6	28.4	22.4	19.7	12.6	7.0	5.1
25-30-9	F14	92.5	61.4	48.5	42.6	27.3	15.3	12.0		57.8	32.5	25.6	22.5	14.4	8.0	5.8
23-30-9	F17 F22	103.1 103.1	76.8 77.3	60.7 68.7	53.3 64.4	34.1 44.3	19.1 24.8	15.1 18.2		72.2 93.9	40.6 52.8	32.1 41.7	28.2 36.6	18.0 23.4	9.6	6.8 7.7
	F27	103.1	77.3	68.7	64.4	51.5	30.0	21.0		103.0	65.0	51.3	45.1	28.8	12.7	8.9
	F34	103.1	77.3	68.7	64.4	51.5	34.8	24.5		103.0	77.2	64.1	56.3	35.1	14.8	10.4
	F8	76.6	43.1	34.0	29.9	19.1	10.7	8.5		49.6	27.9	22.0	19.3	12.3	6.9	5.4
	F11	86.8	60.3	47.6	41.9	26.8	15.0	11.8		69.4	39.0	30.8	27.1	17.3	9.7	7.6
07.00.0	F14	99.9	68.9	54.4	47.8	30.6	17.2	13.5		79.3	44.6	35.2	30.9	19.7	11.0	8.7
27-30-9	F17 F22	111.4 111.4	83.5 83.5	68.1 74.2	59.8 69.5	38.2 49.7	21.4 27.9	16.9 22.0		99.1 111.3	55.7 72.4	44.0 57.2	38.7 50.3	24.7 32.1	13.8 16.8	10.3
	F27	111.4	83.5	74.2	69.6	55.6	34.3	26.0		111.3	83.4	70.4	61.9	39.5	19.4	11.8 13.7
	F34	111.4	83.5	74.2	69.6	55.6	41.7	30.2		111.3	83.4	74.1	69.5	49.4	22.6	15.9
	F8	94.1	61.5	48.6	42.7	27.3	15.3	12.1		76.6	43.1	34.0	29.9	19.1	10.7	8.4
	F11	106.1	79.5	68.0	59.8	38.2	21.5	16.9		106.1	60.3	47.6	41.9	26.8	15.0	11.8
	F14	122.1	91.6	77.7	68.3	43.7	24.5	19.3		122.1	68.9	54.4	47.8	30.6	17.1	13.5
33-30-11	F17	136.1	102.1	90.7	85.0	54.6	30.7	24.2		136.1	86.2	68.1	59.8	38.2	21.4	16.9
	F22	136.1	102.1	90.7	85.0	68.0	39.9	31.5		136.1	102.0	88.5	77.7	49.7	27.9	22.0
	F27 F34	136.1 136.1	102.1	90.7 90.7	85.0 85.0	68.0 68.0	49.1 51.0	38.7		136.1 136.1	102.0 102.0	90.7 90.7	85.0 85.0	61.2 68.0	34.3 42.9	27.0
	г 34	130.1	102.1	90.7	85.0	68.0	51.0	45.3		130.1	102.0	90.7	05.0	00.0	42.9	31.3

Basic Formulae

The following are the basic formulae needed to use the tables.

Force

- A force is simply a push or a pull.
- The standard unit of force is Newton (N).
- Force is often expressed in "kilo-newtons" (kN). 1 kN = 1000 N
- Force (N) = Mass (kg) x Acceleration (ms⁻²)

Weight

- The term "weight" is another way of saying "the force due to gravity". Units are Newtons.
- Gravity is a form of acceleration, and at sea level, is approximately 9.8ms⁻².
- Weight (N) = Mass (kg) x 9.8.

Pressure

- Pressure is the force per unit area, exerted by a force on a surface.
- The standard unit of pressure is Pascals (Pa).
- Pressure is often expressed in "kilo Pascals" (kPa). 1 kPa = 1000Pa
- Pressure = Force (N) / Area (m²)

Example 1 - Calculate the worst case load under each wheel of a forklift.

If a 4 wheeled forklift weights 1000 kg and can lift 2000 kg, and has a worst case weight distribution of 80-20 over the front wheels, the following is how one determines the concentrated loads under each wheel:

Total Weight =
$$(1000 + 2000) \times 9.8$$

= 29400 N = 29.4 kN

Under worst case conditions, 80% of the weight is over the front 2 wheels. Thus, the following is the weight under each front wheel:

= 11.76 kN per wheel

The weight under each rear wheel is:

$$= (29.4 \times 0.2) / 2$$

= 2.94 kN per wheel

Example 2 - Calculate the pressure of a water tank.

If a square water tank is $2m \times 2m$, and contains 3000L of water, calculate the pressure under the water bladder. Assume 1L of water weighs 1 kg.

Area =
$$2 \times 2$$

= $4m^2$

Weight =
$$3000 \times 9.8$$

= 29,400 N = 29.4 kN

= 7350 Pa

= 7.35 kPa

Revision History

Revision	Changes	Date	Who
1	Initial Release		
4	 Complete reformat. Changed load span table layout, and added more constructions. Modified Fixings section. Added "Basic Formulae" section. Added a section to aid in interpreting the tables. Added a section on formaldehyde. 	04-11-2008	МВ

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